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Title: Los Alamos Seismology and Seismic Retrofit of Mission Critical

Facilities

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Los Alamos Seismology and Seismic Retrofit of Mission Critical Facilities

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Presentation Major Points -

- Discussion on the seismic hazard in Los Alamos, New Mexico
 - Inputs From Geology and Seismology
- Ongoing Seismic Retrofit of Los Alamos National Laboratory (LANL) Plutonium Research Facility.
 - Column Capital Test Program (at University of Nevada Reno)

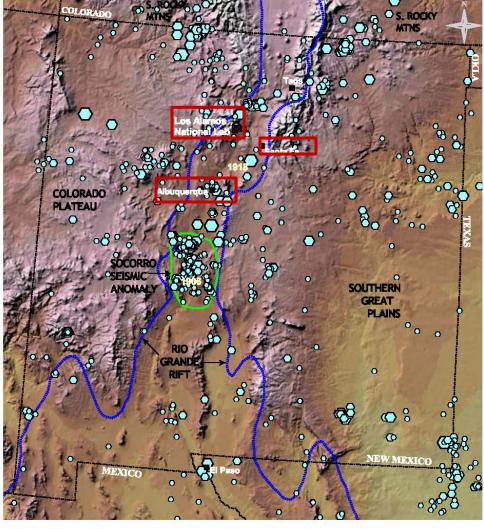




Seismicity in New Mexico

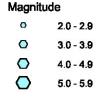
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Historical Seismicity, 1869 to 2005







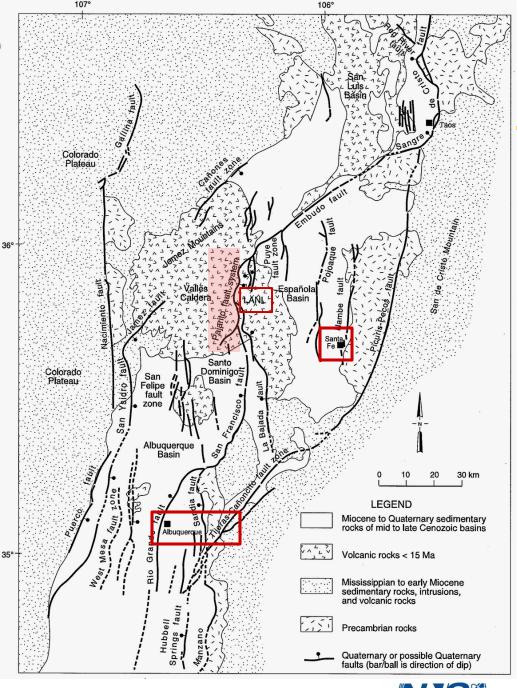








Faulting Map of the Los Alamos Region







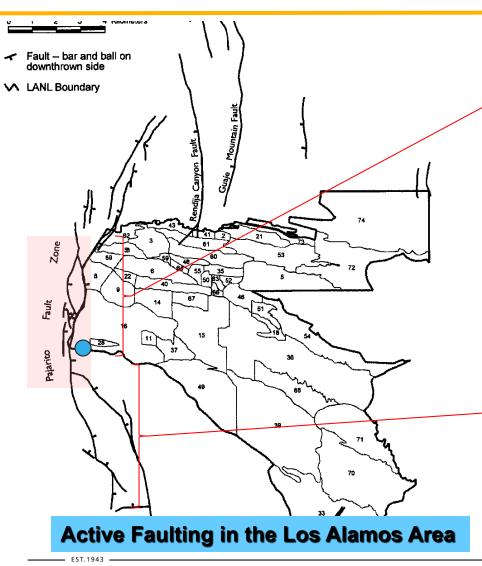
Aerial View of LANL and Jemez Mountains





Looking West

Faults at LANL





Looking North



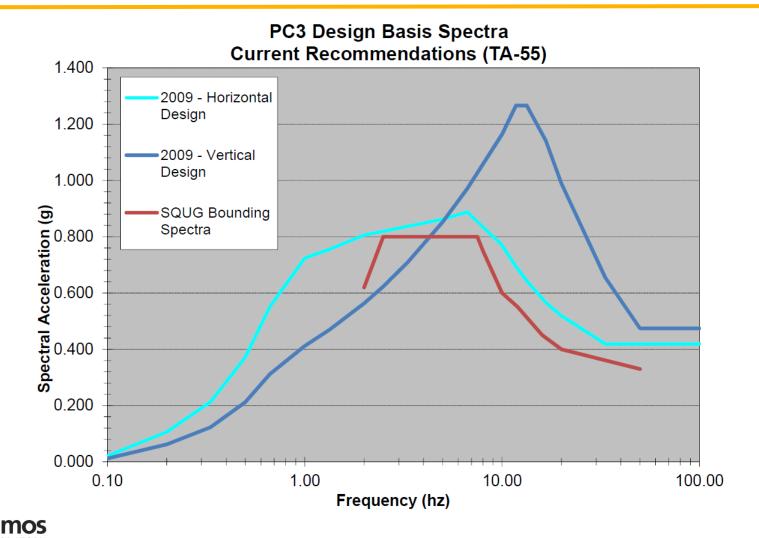


Fault Investigation





TA-55 Free Field Response Spectra





Seismic Retrofitting LANL Critical Facilities

Column Capital Test Program (CCTP)





Column Capital Test Program (CCTP)

- Full-scale testing effort being carried out at University of Nevada – Reno to assess capacity of unreinforced column capitals.
- Goal to understand the fragility of the unreinforced capitals under pseudo-static loading representative of LANL seismic hazard.





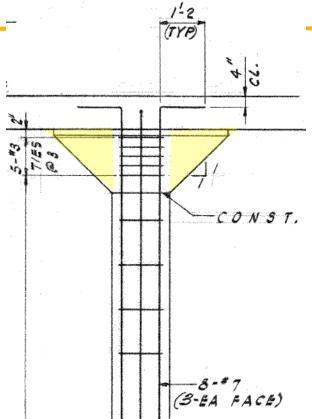
LANL Plutonium Research Facility (PF-4)

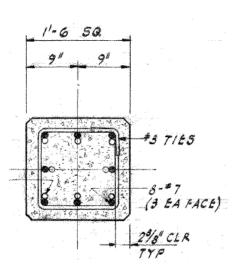




'Captured' Column in PF-4







Concern is loss of unreinforced capital leading to reduced punching shear capacity.

Seismic Retrofit of 'Captured' Columns

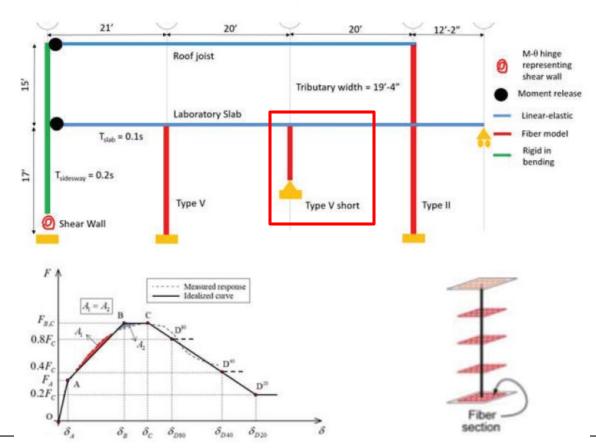


Column jacketed with 13 layers of BASF Carbon Fiber Reinforced Polymer (CFRP).

CFRP jacketing will prevent shear failure due to ties that terminate in 90-degree hooks.

CFRP jacketing will increase concrete strain ε_c at failure from 0.003 to 0.01 to increase drift capacity under seismic event.

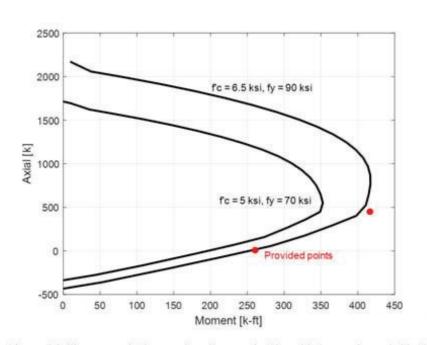
Develop 2D model representing major components in building to define Axial (P) – Moment (M) phasing on Captured Columns







Results from modeling effort to define P-M interaction and loading on Captured Column



300 200 100 100 -200 -300 -300 -400 -8 -6 -4 -2 0 2 4 6 8 10 12 Curvature [rad/in] ×10⁻⁴

Figure 37. Moment-axial interaction diagram for Type V short column (with fiber-wrap).

Figure 38. Sample moment-curvature response for a Type V short column subjected to different intensities of the same ground motion.





Recommendations on Loading Protocol:

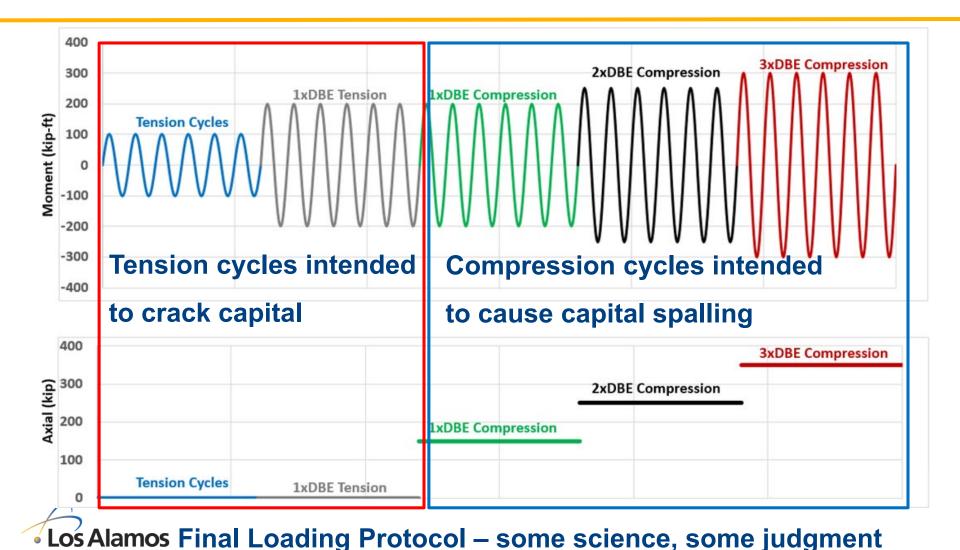
Rounded/Adjusted Envelope Type V Protocol

	median				90%ile			
	M.	P ⁻	M⁺	P⁺	M ⁻	P ⁻	M⁺	P⁺
1×DBE	200	100	200	200	225	65	223	160
2×DBE	225	75	230	230	250	30	260	250
3×DBE	250	50	260	250	300	0	300	350

DBE – Design Basis Event (EQ with 2,500 year reoccurrence period). Performance above 3 x DBE implies capital failure will not contribute to combined probability of failure of PF-4.







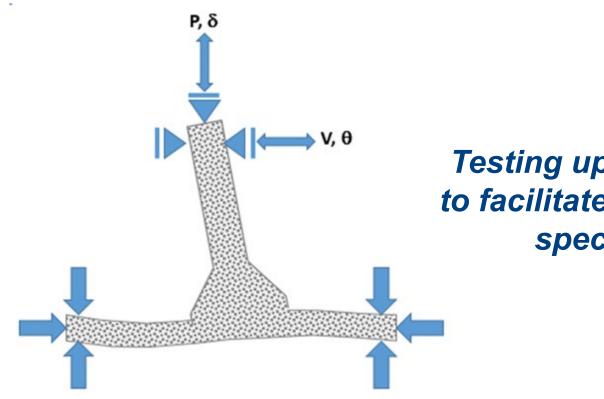
Column Capital Test Program

Specimen Development





Column Capital Test Program (CCTP) Pre-testing Modeling and Evaluation



Testing upside down to facilitate stability of specimen

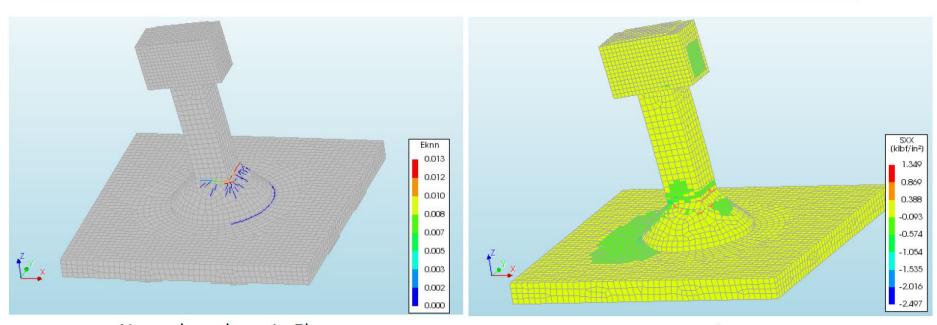


Schematic of Specimen Test as Defined in Contract Documents



Column Capital Test Program (CCTP) Pre-testing Modeling and Evaluation

Nonlinear Load History (Case 6 and $f_c = 4$ ksi)



Normal crack strain Eknn

Normal stress σ_{x}

Modeling to Refine Specimen by Dr. Suiwen Wu of UNR Using DIANA

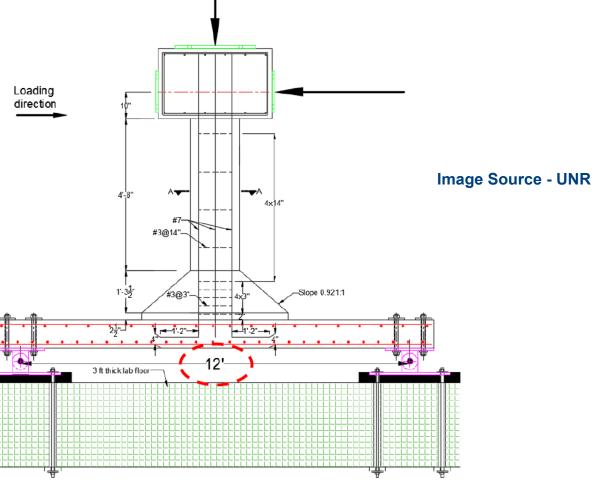


Image Source – University of Nevada – Reno (UNR)

Column Capital Test Program (CCTP) Pretesting Modeling and Evaluation

 13.5'x13.5' plan dimensions for slab

 Center-to-center distance of pin supports is 12'

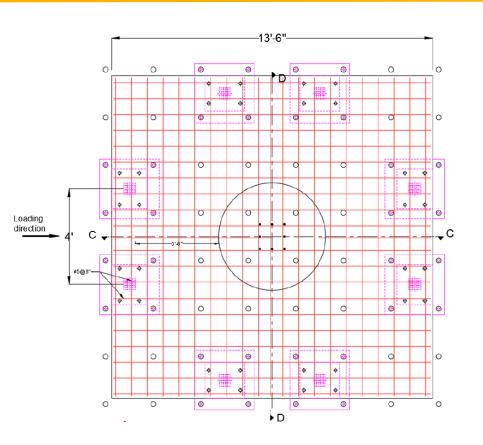


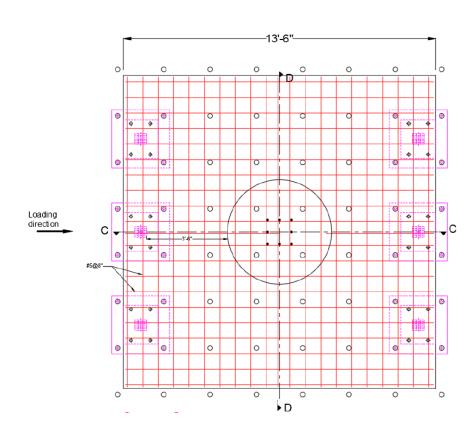


Development of Specimen



Column Capital Test Program (CCTP) Pre-testing Modeling and Evaluation





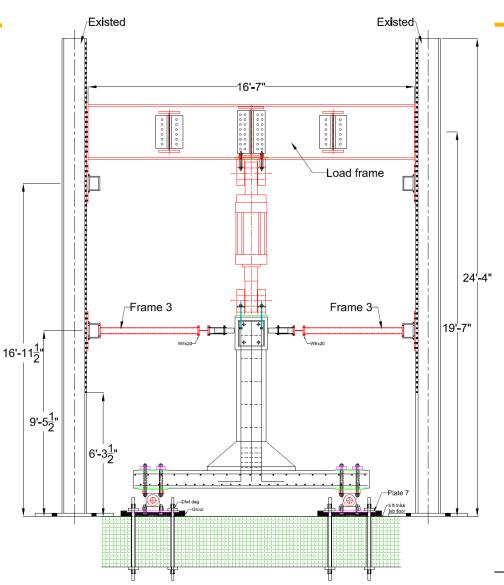
Development of Specimen – Support Configuration



Image Source - UNR



Column Capital Test Program (CCTP) Final Specimen Configuration



Schematic of Load Frame and Specimen

Image Source - UNR

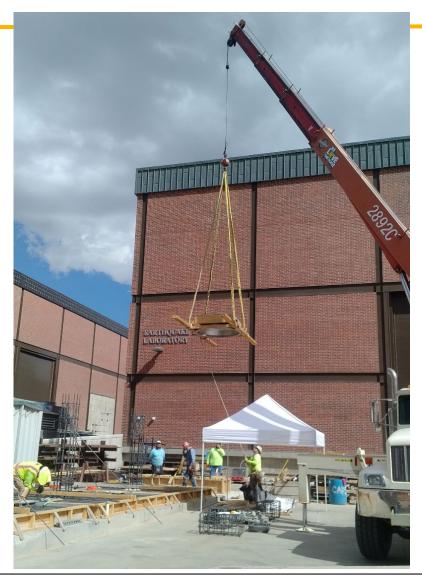


Column Capital Test Program

Specimen Construction At University of Nevada - Reno



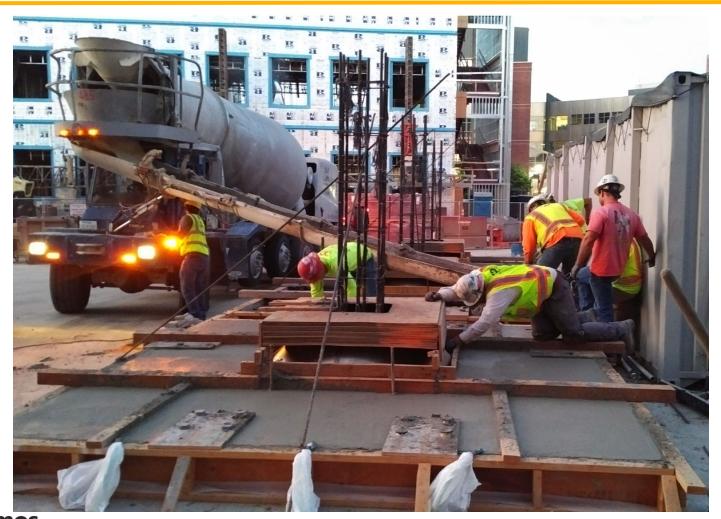




Final Rebar Configuration July 23, 2019



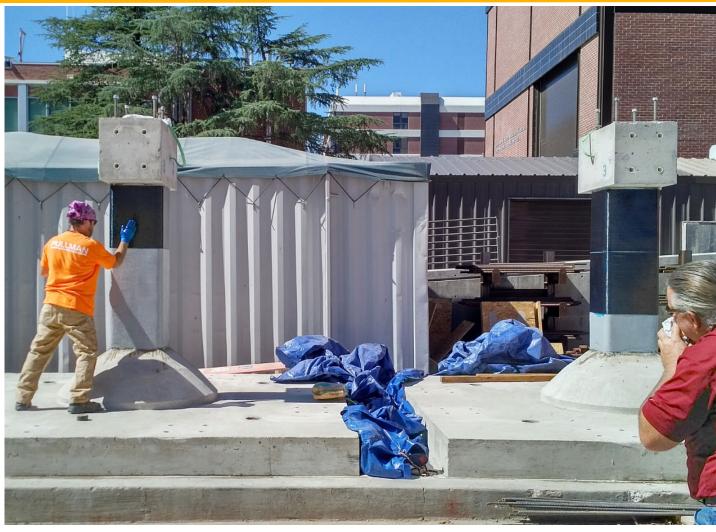








CFRP Installation September 2019







Specimens Following Installation of CFRP



Image Source - UNR



Column Capital Test Program (CCTP) Specimen Test Model Setup



Specimen 1
Before Testing





Column Capital Test Program

Specimen Testing





Column Capital Test Program (CCTP) Specimen Testing



Specimen 1 at
Failure
Load Step 8
~6% drift

$$P = 0 k$$

$$M = Mp \sim = 400 \text{ ft-k}$$



Column Capital Test Program (CCTP) Specimen Testing



Specimen 2 at Slab Punching Shear Failure Load Step 6

$$P \sim = 600 \text{ k}$$
 $M = M_{P-\Delta} \sim = 50 \text{ ft-k}$





Column Capital Test Program (CCTP) Specimen Testing





Specimen 4 at Failure
Load Step 9

$$P = \sim 510 \text{ k}$$

$$M = M_{P-\Delta} \sim = 50 \text{ ft-k}$$



Column Capital Test Program (CCTP) Conclusion

- The unreinforced capitals are incredibly robust.
- Behavior was mostly linear over full loading protocol.
- Punching shear strength will not be diminished.
- FEA could not predict capacity. Grossly in error.





LANL/UNR CCTP and Los Alamos Seismology

Questions?

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